



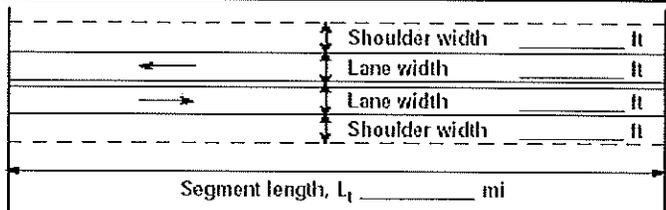
# Appendix 6

## Operational Analysis Worksheets



# Appendix 6

## Existing Two-Lane Highway 2012

DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET			
<b>General Information</b>		<b>Site Information</b>	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	RP 0.6 to 20.0 NB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2012
Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<b>Input Data</b>			
		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway <input type="checkbox"/> Class III highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Grade Length mi    Up/down Peak-hour factor, PHF    0.81 No-passing zone    27% % Trucks and Buses, P <sub>T</sub> 27% % Recreational vehicles, P <sub>R</sub> 4% Access points mi    5/mi	
Analysis direction vol., V <sub>d</sub>	135veh/h	 Show North Arrow	
Opposing direction vol., V <sub>o</sub>	139veh/h		
Shoulder width ft	8.0		
Lane Width ft	12.0		
Segment Length mi	19.4		
<b>Average Travel Speed</b>			
		Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, E <sub>T</sub> (Exhibit 15-11 or 15-12)		1.6	1.6
Passenger-car equivalents for RVs, E <sub>R</sub> (Exhibit 15-11 or 15-13)		1.0	1.0
Heavy-vehicle adjustment factor, f <sub>HV,ATS</sub> = 1 / (1 + P <sub>T</sub> (E <sub>T</sub> -1) + P <sub>R</sub> (E <sub>R</sub> -1))		0.861	0.861
Grade adjustment factor <sup>1</sup> , f <sub>g,ATS</sub> (Exhibit 15-9)		1.00	1.00
Demand flow rate <sup>2</sup> , v <sub>i</sub> (pc/h) v <sub>i</sub> = V <sub>i</sub> / (PHF * f <sub>g,ATS</sub> * f <sub>HV,ATS</sub> )		194	199
<b>Free-Flow Speed from Field Measurement</b>		<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , S <sub>FM</sub>		Base free-flow speed <sup>4</sup> , BFFS	65.0 mi/h
Total demand flow rate, both directions, v		Adj. for lane and shoulder width <sup>4</sup> , f <sub>LS</sub> (Exhibit 15-7)	0.0 mi/h
Free-flow speed, FFS = S <sub>FM</sub> + 0.00776(v / f <sub>HV,ATS</sub> )		Adj. for access points <sup>4</sup> , f <sub>A</sub> (Exhibit 15-8)	1.3 mi/h
Adj. for no-passing zones, f <sub>np,ATS</sub> (Exhibit 15-15)	2.5 mi/h	Free-flow speed, FFS (FSS = BFFS - f <sub>LS</sub> - f <sub>A</sub> )	63.8 mi/h
		Average travel speed, ATS <sub>d</sub> = FFS - 0.00776(v <sub>d,ATS</sub> + v <sub>o,ATS</sub> ) - f <sub>np,ATS</sub>	58.2 mi/h
		Percent free flow speed, PFFS	91.3 %
<b>Percent Time-Spent-Following</b>			
		Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, E <sub>T</sub> (Exhibit 15-18 or 15-19)		1.1	1.1
Passenger-car equivalents for RVs, E <sub>R</sub> (Exhibit 15-18 or 15-19)		1.0	1.0
Heavy-vehicle adjustment factor, f <sub>HV</sub> = 1 / (1 + P <sub>T</sub> (E <sub>T</sub> -1) + P <sub>R</sub> (E <sub>R</sub> -1))		0.974	0.974
Grade adjustment factor <sup>1</sup> , f <sub>g,PTSF</sub> (Exhibit 15-16 or Ex 15-17)		1.00	1.00
Directional flow rate <sup>2</sup> , v <sub>i</sub> (pc/h) v <sub>i</sub> = V <sub>i</sub> / (PHF * f <sub>HV,PTSF</sub> * f <sub>g,PTSF</sub> )		171	176
Base percent time-spent-following <sup>4</sup> , BPTSF <sub>d</sub> (%) = 100(1 - e <sup>-av<sub>d</sub></sup> )		18.8	
Adj. for no-passing zone, f <sub>np,PTSF</sub> (Exhibit 15-21)		42.2	
Percent time-spent-following, PTSF <sub>d</sub> (%) = BPTSF <sub>d</sub> + f <sub>np,PTSF</sub> * (v <sub>d,PTSF</sub> / v <sub>d,PTSF</sub> + v <sub>o,PTSF</sub> )		39.6	
<b>Level of Service and Other Performance Measures</b>			
Level of service, LOS (Exhibit 15-3)		B	
Volume to capacity ratio, v/c		0.13	

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1464
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1655
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	91.3
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	166.7
Effective width, $Wv$ (Eq. 15-29) ft	34.50
Effective speed factor, $S_t$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	11.34
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o</math> <math>\geq</math> 1,700 pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	RP 0.6 to RP 12.4 SB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2012

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway     Class II highway

Class III highway

Terrain     Level     Rolling

Grade Length mi    Up/down

Peak-hour factor, PHF    0.78

No-passing zone    20%

% Trucks and Buses,  $P_T$     29%

% Recreational vehicles,  $P_R$     4%

Access points mi    7/mi

Analysis direction vol., $V_d$	139veh/h
Opposing direction vol., $V_o$	135veh/h
Shoulder width ft	8.0
Lane Width ft	12.0
Segment Length mi	11.8

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.6	1.6
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.852	0.852
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	209	203

Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS    65.0 mi/h
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)    0.0 mi/h
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)    1.8 mi/h
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    2.1 mi/h	Free-flow speed, FFS ( $FFS = BFFS * f_{LS} * f_A$ )    63.3 mi/h
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 58.0 mi/h
	Percent free flow speed, PFFS    91.6 %

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.972	0.972
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	183	178
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(%) = 100(1 - e^{-av_d^b})$	20.0	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	38.4	
Percent time-spent-following, $PTSF_d(%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	39.5	

**Level of Service and Other Performance Measures**

Level of service, LOS (Exhibit 15-3)	B
Volume to capacity ratio, $v/c$	0.14

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1448
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1652
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	91.6
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	178.2
Effective width, $Wv$ (Eq. 15-29) ft	34.10
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	13.06
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> (or <math>v_o</math>) <math>\geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

<b>DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET WITH PASSING LANE WORKSHEET</b>	
<b>General Information</b>	
Analyst	David Stoner
Agency or Company	DOWL HKM
Date Performed	4/17/2012
Analysis Time Period	Peak Hour
<b>Site Information</b>	
Highway of Travel	MT 16
From/To	RP 20.0 to Savage NB
Jurisdiction	Dawson/Richland County
Analysis Year	2012
Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study	
<b>Input Data</b>	
<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway <input type="checkbox"/> Class III highway	
Shoulder width (ft)	8.0
Lane Width (ft)	12.0
Segment Length (mi)	11.5
Total length of analysis segment, $L_t$	11.5
Length of two-lane highway upstream of the passing lane, $L_u$	0.0
Length of passing lane including tapers, $L_{pl}$	1.9
Average travel speed, $ATS_d$ (from Directional Two-Lane Highway Segment Worksheet)	58.6
Percent time-spent-following, $PTSF_d$ (from Directional Two-Lane Highway Segment Worksheet)	37.3
Level of service <sup>1</sup> , $LOS_d$ (from Directional Two-Lane Highway Segment Worksheet)	B
<b>Average Travel Speed</b>	
Length of the downstream highway segment within the effective length of passing lane for average travel speed, $L_{de}$ (Exhibit 15-23)	1.70
Length of two-lane highway downstream of effective length of the passing lane for avg travel speed, $L_d = L_t - (L_u + L_{pl} + L_{de})$	7.90
Adj. factor for the effect of passing lane on average speed, $f_{pl}$ (Exhibit 15-28)	1.08
Average travel speed including passing lane <sup>2</sup> , $ATS_{pl} = (ATS_d * L_t) / (L_u + L_d + (L_{pl}/f_{pl}) + (2L_{de}/(1+f_{pl}ATS)))$	59.7
Percent free flow speed including passing lane, $FFFS_{pl} = (ATS_{pl}/FFS)$	93.6
<b>Percent Time-Spent-Following</b>	
Length of the downstream highway segment within the effective length of passing lane for percent time-spent-following, $L_{de}$ (Exhibit 15-23)	13.00
Length of two-lane highway downstream of effective length of the passing lane for percent-time-following, $L_d = L_t - (L_u + L_{pl} + L_{de})$	-3.40
Adj. factor for the effect of passing lane on percent time-spent-following, $f_{pl,PTSF}$ (Exhibit 15-26)	0.58

Percent time-spent-following including passing lane <sup>3</sup> , $PTSF_{pl}(\%)$ $PTSF_{pl} = PTSF_d [L_u + L_d + f_{pl} PTSF_{pl} + ((1 + f_{pl} PTSF_{pl}) / 2) L_{de}] / L_t$	26.5
<b>Level of Service and Other Performance Measures<sup>4</sup></b>	
Level of service including passing lane $LOS_{pl}$ (Exhibit 15-3)	A
Peak 15-min total travel time, $TT_{15}(\text{veh-h})$ $TT_{15} = VMT_{15} / ATS_{pl}$	8.0
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	166.7
Effective width, $W_v$ (Eq. 15-29) ft	34.50
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, $BLOS$ (Eq. 15-31)	11.34
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. If <math>LOS_d = F</math>, passing lane analysis cannot be performed.</p> <p>2. If <math>L_d &lt; 0</math>, use alternative Equation 15-18.</p> <p>3. If <math>L_d &lt; 0</math>, use alternative Equation 15-16.</p> <p>4. <math>v/c</math>, <math>VMT_{15}</math> and <math>VMT_{60}</math> are calculated on Directional Two-Lane Highway Segment Worksheet.</p>	

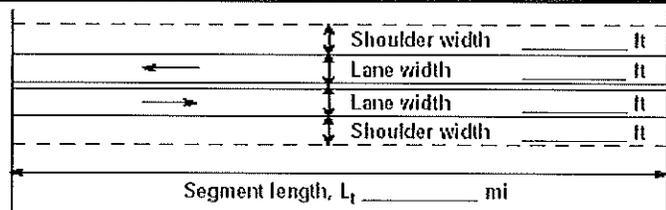
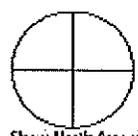
<b>DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET WITH PASSING LANE WORKSHEET</b>	
<b>General Information</b>	
Analyst Agency or Company Date Performed Analysis Time Period	David Stoner DOWL HKM 4/17/2012 Peak Hour
<b>Site Information</b>	
Highway of Travel From/To Jurisdiction Analysis Year	MT 16 RP 12.4 to RP 22.0 SB Dawson/Richland County 2012
Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study	
<b>Input Data</b>	
<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway <input type="checkbox"/> Class III highway	
Shoulder width (ft)	8.0
Lane Width (ft)	12.0
Segment Length (mi)	9.6
Total length of analysis segment, $L_t$	9.6
Length of two-lane highway upstream of the passing lane, $L_u$	0.0
Length of passing lane including tapers, $L_{pi}$	1.9
Average travel speed, $ATS_d$ (from Directional Two-Lane Highway Segment Worksheet)	59.1
Percent time-spent-following, $PTSF_d$ (from Directional Two-Lane Highway Segment Worksheet)	37.1
Level of service <sup>1</sup> , $LOS_d$ (from Directional Two-Lane Highway Segment Worksheet)	B
<b>Average Travel Speed</b>	
Length of the downstream highway segment within the effective length of passing lane for average travel speed, $L_{de}$ (Exhibit 15-23)	1.70
Length of two-lane highway downstream of effective length of the passing lane for avg travel speed, $L_d = L_t - (L_u + L_{pi} + L_{de})$	6.00
Adj. factor for the effect of passing lane on average speed, $f_{pl}$ (Exhibit 15-28)	1.09
Average travel speed including passing lane <sup>2</sup> , $ATS_{pi} = (ATS_d * L_t) / (L_u + L_d + (L_{pi}/f_{pl}) + (2L_{de}/(1+f_{pl}ATS_d)))$	60.6
Percent free flow speed including passing lane, $PFFS_{pi} = (ATS_{pi}/FFS)$	93.9
<b>Percent Time-Spent-Following</b>	
Length of the downstream highway segment within the effective length of passing lane for percent time-spent-following, $L_{de}$ (Exhibit 15-23)	13.00
Length of two-lane highway downstream of effective length of the passing lane for percent-time-following, $L_d = L_t - (L_u + L_{pi} + L_{de})$	-5.30
Adj. factor for the effect of passing lane on percent time-spent-following, $f_{pl,PTSF}$ (Exhibit 15-26)	0.58

Percent time-spent-following including passing lane <sup>3</sup> , $PTSF_{pl}(\%)$ $PTSF_{pl} = PTSF_d [L_u + L_d + f_{pl, PTSF} L_{pl} + ((1 + f_{pl, PTSF})/2) L_{de}] / L_t$	25.2
<b>Level of Service and Other Performance Measures<sup>4</sup></b>	
Level of service including passing lane $LOS_{pl}$ (Exhibit 15-3)	A
Peak 15-min total travel time, $TT_{15}(\text{veh-h})$ $TT_{15} = VMT_{15}/ATS_{pl}$	7.1
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	178.2
Effective width, $W_v$ (Eq. 15-29) ft	34.10
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	13.06
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
1. If $LOS_d = F$ , passing lane analysis cannot be performed. 2. If $L_d < 0$ , use alternative Equation 15-18. 3. If $L_d < 0$ , use alternative Equation 15-16. 4. $v/c$ , $VMT_{15}$ and $VMT_{60}$ are calculated on Directional Two-Lane Highway Segment Worksheet.	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	RP 22.0 to Savage SB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2012

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

Input Data	
 <p>Shoulder width _____ ft</p> <p>Lane width _____ ft</p> <p>Lane width _____ ft</p> <p>Shoulder width _____ ft</p> <p>Segment length, <math>L_1</math> _____ mi</p> <p>Analysis direction vol., <math>V_d</math>      139veh/h</p> <p>Opposing direction vol., <math>V_o</math>      135veh/h</p> <p>Shoulder width ft                      8.0</p> <p>Lane Width ft                            12.0</p> <p>Segment Length mi                    9.5</p>	<div style="display: flex; justify-content: space-between;"> <div style="text-align: center;"> <input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway  <input type="checkbox"/> Class III highway                 </div> <div style="text-align: center;"> <input checked="" type="checkbox"/> Level                      <input type="checkbox"/> Rolling                      Terrain                 </div> </div> <p>Grade Length mi                      Up/down</p> <p>Peak-hour factor, PHF                0.78</p> <p>No-passing zone                        22%</p> <p>% Trucks and Buses, <math>P_T</math>            29 %</p> <p>% Recreational vehicles, <math>P_R</math>        4%</p> <p>Access points mi                        5/mi</p> <div style="text-align: center;">                       Show North Arrow                 </div>

Average Travel Speed		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.6	1.6
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.852	0.852
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	209	203
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed
Mean speed of sample <sup>3</sup> , $S_{FM}$		Base free-flow speed <sup>4</sup> , BFFS                      65.0 mi/h
Total demand flow rate, both directions, $v$		Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)      0.0 mi/h
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$		Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)                      1.3 mi/h
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)                      2.2 mi/h		Free-flow speed, FFS ( $FFS = BFFS * f_{LS} * f_A$ )                      63.8 mi/h
		Average travel speed, $ATS_d = FFS - 0.00776(v_d,ATS + v_o,ATS) * f_{np,ATS}$ 58.3 mi/h
		Percent free flow speed, PFFS    91.5 %

Percent Time-Spent-Following		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.972	0.972
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	183	178
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(%) = 100(1 - e^{-a v_d^b})$		20.0
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)		39.7
Percent time-spent-following, $PTSF_d(%) = BPTSF_d + f_{np,PTSF} * (v_d,PTSF / v_o,PTSF)$		40.1

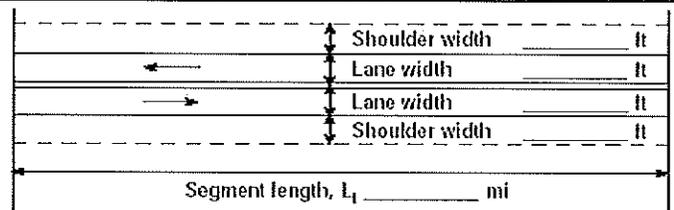
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 15-3)	B
Volume to capacity ratio, $v/c$	0.14

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1448
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1652
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	91.5
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	178.2
Effective width, $W_v$ (Eq. 15-29) ft	34.10
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	13.06
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o</math> <math>\geq</math> 1,700 pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	Savage to Crane NB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2012

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

Input Data	
 <p>Shoulder width _____ ft</p> <p>Lane width _____ ft</p> <p>Lane width _____ ft</p> <p>Shoulder width _____ ft</p> <p>Segment length, <math>L_1</math> _____ mi</p> <p>Analysis direction vol., <math>V_d</math> 141veh/h</p> <p>Opposing direction vol., <math>V_o</math> 171veh/h</p> <p>Shoulder width ft 8.0</p> <p>Lane Width ft 12.0</p> <p>Segment Length mi 10.0</p>	<div style="display: flex; justify-content: space-between;"> <div style="text-align: center;">  <p>Show North Arrow</p> </div> <div> <input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway  <input type="checkbox"/> Class III highway                  Terrain    <input checked="" type="checkbox"/> Level    <input type="checkbox"/> Rolling                  Grade Length mi    Up/down                  Peak-hour factor, PHF 0.87                  No-passing zone 31%                  % Trucks and Buses, <math>P_T</math> 23 %                  % Recreational vehicles, <math>P_R</math> 4%                  Access points mi 11/mi             </div> </div>

Average Travel Speed		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.7	1.5
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.861	0.897
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{g,ATS} * f_{HV,ATS})$	188	219
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed
Mean speed of sample <sup>3</sup> , $S_{FM}$		Base free-flow speed <sup>4</sup> , BFFS 69.0 mi/h
Total demand flow rate, both directions, $v$		Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7) 0.0 mi/h
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$		Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8) 2.8 mi/h
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15) 2.7 mi/h		Free-flow speed, FFS ( $FFS = BFFS * f_{LS} * f_A$ ) 66.3 mi/h
		Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 60.4 mi/h
		Percent free flow speed, PFFS 91.1 %

Percent Time-Spent-Following		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.978	0.978
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	166	201
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(%) = 100(1 - e^{-a v_d^b})$		18.3
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)		43.4
Percent time-spent-following, $PTSF_d(%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$		37.9

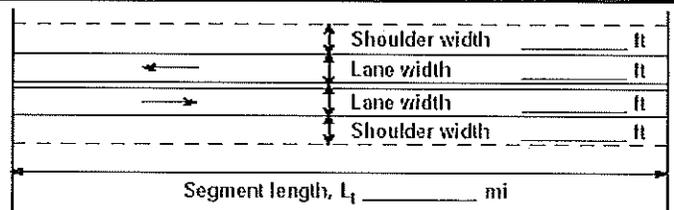
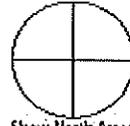
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 15-3)	B
Volume to capacity ratio, v/c	0.12

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1525
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1662
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	91.1
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	162.1
Effective width, $W_v$ (Eq. 15-29) ft	33.90
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	8.67
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<ol style="list-style-type: none"> <li>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</li> <li>2. If <math>v_d</math> or <math>v_o</math> <math>\geq 1,700</math> pc/h, terminate analysis--the LOS is F.</li> <li>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</li> <li>4. For the analysis direction only</li> <li>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</li> <li>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</li> </ol>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	Savage to Crane SB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2012

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

Input Data	
 <p>Shoulder width _____ ft</p> <p>Lane width _____ ft</p> <p>Lane width _____ ft</p> <p>Shoulder width _____ ft</p> <p>Segment length, <math>L_1</math> _____ mi</p> <p>Analysis direction vol., <math>V_d</math>      171veh/h</p> <p>Opposing direction vol., <math>V_o</math>      141veh/h</p> <p>Shoulder width ft                      8.0</p> <p>Lane Width ft                            12.0</p> <p>Segment Length mi                    10.0</p>	<div style="text-align: center;">  <p>Show North Arrow</p> </div> <p><input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway</p> <p><input type="checkbox"/> Class III highway</p> <p>Terrain    <input checked="" type="checkbox"/> Level    <input type="checkbox"/> Rolling</p> <p>Grade Length mi    Up/down</p> <p>Peak-hour factor, PHF      0.84</p> <p>No-passing zone                19%</p> <p>% Trucks and Buses, <math>P_T</math>      25 %</p> <p>% Recreational vehicles, <math>P_R</math>    4%</p> <p>Access points mi                11/mi</p>

Average Travel Speed		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.5	1.6
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.889	0.870
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	229	193
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed
Mean speed of sample <sup>3</sup> , $S_{FM}$		Base free-flow speed <sup>4</sup> , BFFS      66.0 mi/h
Total demand flow rate, both directions, $v$		Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)    0.0 mi/h
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$		Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)      2.8 mi/h
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)      2.0 mi/h		Free-flow speed, FFS ( $FSS = BFFS * f_{LS} * f_A$ )      63.3 mi/h
		Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 58.0 mi/h
		Percent free flow speed, PFFS      91.6 %

Percent Time-Spent-Following		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.976	0.976
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	209	172
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(%) = 100(1 - e^{-av_d^b})$		22.4
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)		36.7
Percent time-spent-following, $PTSF_d(%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$		42.5

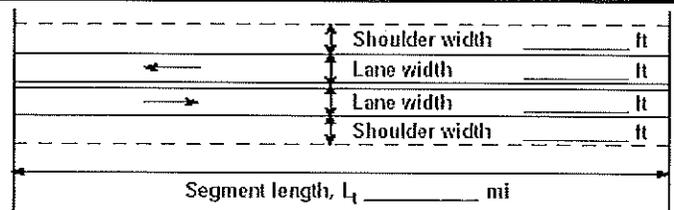
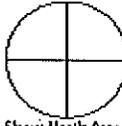
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 15-3)	B
Volume to capacity ratio, v/c	0.15

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1479
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1659
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	91.6
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	203.6
Effective width, $Wv$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	12.00
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o</math> <math>\geq</math> 1,700 pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	Crane to Sidney NB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2012

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

Input Data	
 <p style="margin-left: 20px;">Shoulder width _____ ft Lane width _____ ft Lane width _____ ft Shoulder width _____ ft</p> <p style="margin-left: 20px;">Segment length, <math>L_1</math> _____ mi</p>	<div style="display: flex; justify-content: space-between;"> <div style="text-align: center;">               Show North Arrow         </div> <div> <input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway  <input type="checkbox"/> Class III highway              Terrain    <input checked="" type="checkbox"/> Level    <input type="checkbox"/> Rolling              Grade Length    mi    Up/down              Peak-hour factor, PHF    0.80              No-passing zone    24%              % Trucks and Buses, <math>P_T</math>    19%              % Recreational vehicles, <math>P_R</math>    4%              Access points    mi    12/mi         </div> </div>
Analysis direction vol., $V_d$ 151veh/h	
Opposing direction vol., $V_o$ 232veh/h	
Shoulder width ft    8.0	
Lane Width ft    12.0	
Segment Length mi    8.9	

Average Travel Speed		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.5	1.4
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.913	0.929
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{g,ATS} * f_{HV,ATS})$	207	312
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed
Mean speed of sample <sup>3</sup> , $S_{FM}$		Base free-flow speed <sup>4</sup> , BFFS    65.0 mi/h
Total demand flow rate, both directions, $v$		Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)    0.0 mi/h
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$		Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)    3.0 mi/h
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    1.9 mi/h		Free-flow speed, FFS ( $FFS = BFFS * f_{LS} * f_A$ )    62.0 mi/h
		Average travel speed, $ATS_d = FFS - 0.00776(v_d / f_{HV,ATS} + v_o / f_{HV,ATS}) - f_{np,ATS}$ 56.1 mi/h
		Percent free flow speed, PFFS    90.5 %

Percent Time-Spent-Following		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.981	0.981
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	192	296
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(%) = 100(1 - e^{-a v_d^b})$	23.2	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	37.7	
Percent time-spent-following, $PTSF_d(%) = BPTSF_d + f_{np,PTSF} * (v_d / v_o)$	38.0	

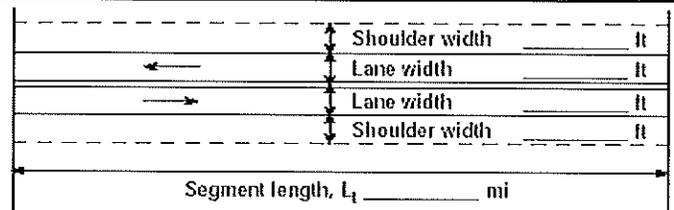
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 15-3)	B
Volume to capacity ratio, $v/c$	0.13

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1579
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1668
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	90.5
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	188.8
Effective width, $Wv$ (Eq. 15-29) ft	32.90
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	6.55
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o</math> <math>\geq</math> 1,700 pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	Crane to Sidney SB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2012

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

Input Data	
 <p>Shoulder width _____ ft</p> <p>Lane width _____ ft</p> <p>Lane width _____ ft</p> <p>Shoulder width _____ ft</p> <p>Segment length, <math>L_1</math> _____ mi</p> <p>Analysis direction vol., <math>V_d</math>      232veh/h</p> <p>Opposing direction vol., <math>V_o</math>      151veh/h</p> <p>Shoulder width ft                      8.0</p> <p>Lane Width ft                            12.0</p> <p>Segment Length mi                    8.9</p>	<div style="text-align: center;">  <p>Show North Arrow</p> </div> <p><input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway</p> <p><input type="checkbox"/> Class III highway</p> <p>Terrain    <input checked="" type="checkbox"/> Level    <input type="checkbox"/> Rolling</p> <p>Grade Length mi                      Up/down</p> <p>Peak-hour factor, PHF                0.87</p> <p>No-passing zone                        22%</p> <p>% Trucks and Buses, <math>P_T</math>            19 %</p> <p>% Recreational vehicles, <math>P_R</math>      4%</p> <p>Access points mi                        12/mi</p>

Average Travel Speed		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.4	1.6
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.929	0.898
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{g,ATS} * f_{HV,ATS})$	287	193
<b>Free-Flow Speed from Field Measurement</b>		<b>Estimated Free-Flow Speed</b>
Mean speed of sample <sup>3</sup> , $S_{FM}$		Base free-flow speed <sup>4</sup> , BFFS                      69.0 mi/h
Total demand flow rate, both directions, $v$		Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)      0.0 mi/h
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$		Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)                      3.0 mi/h
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)                      2.2 mi/h		Free-flow speed, FFS ( $FFS = BFFS - f_{LS} - f_A$ )                      66.0 mi/h
		Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 60.0 mi/h
		Percent free flow speed, PFFS    91.0 %

Percent Time-Spent-Following		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.981	0.981
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	272	177
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(%) = 100(1 - e^{-a v_d^b})$		27.9
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)		36.8
Percent time-spent-following, $PTSF_d(%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$		50.2

Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, $v/c$	0.19

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1527
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1668
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	91.0
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	266.7
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	8.22
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_i(v_d \text{ or } v_o) \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 200
Agency or Company	DOWL HKM	From/To	Sidney to Fairview EB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2012

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway     Class II highway

Class III highway

Terrain     Level     Rolling

Grade Length mi    Up/down

Peak-hour factor, PHF    0.83

No-passing zone    17%

% Trucks and Buses,  $P_T$     17%

% Recreational vehicles,  $P_R$     4%

Access points mi    16/mi

Analysis direction vol., $V_d$	257veh/h
Opposing direction vol., $V_o$	254veh/h
Shoulder width ft	8.0
Lane Width ft	12.0
Segment Length mi	9.9

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.4	1.4
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.936	0.936
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	331	327
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS    69.0 mi/h	
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)    0.0 mi/h	
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)    4.0 mi/h	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    1.8 mi/h	Free-flow speed, FFS ( $FSS = BFFS * f_{LS} * f_A$ )    65.0 mi/h	
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 58.1 mi/h	
	Percent free flow speed, PFFS    89.3 %	

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.983	0.983
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	315	311
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-av_d^b})$	33.8	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	34.3	
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	51.1	

**Level of Service and Other Performance Measures**

Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, v/c	0.21

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1591
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1672
Percent Free-Flow Speed $PF_{FS_d}$ (Equation 15-11 - Class III only)	89.3
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	309.6
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	7.15
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_1(v_d \text{ or } v_o) \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 200
Agency or Company	DOWL HKM	From/To	Sidney to Fairview WB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2012

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway     Class II highway

Class III highway

Terrain     Level     Rolling

Grade Length mi    Up/down

Peak-hour factor, PHF    0.86

No-passing zone    15%

% Trucks and Buses,  $P_T$     25%

% Recreational vehicles,  $P_R$     4%

Access points mi    16/mi

Analysis direction vol., $V_d$	254veh/h		
Opposing direction vol., $V_o$	257veh/h		
Shoulder width ft	8.0		
Lane Width ft	12.0		
Segment Length mi	9.9		

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.4	1.4
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.909	0.909
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	325	329
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$		66.0 mi/h
Total demand flow rate, both directions, $v$		0.0 mi/h
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$		4.0 mi/h
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    1.7 mi/h		62.0 mi/h
		Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$
		Percent free flow speed, PFFS    89.1 %

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.976	0.976
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	303	306
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-a v_d^b})$		33.2
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)		32.4
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$		49.3

**Level of Service and Other Performance Measures**

Level of service, LOS (Exhibit 15-3)	B
Volume to capacity ratio, $v/c$	0.21

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1545
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1659
Percent Free-Flow Speed $PF_{FS_d}$ (Equation 15-11 - Class III only)	89.1
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	295.3
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	12.19
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_1(v_d \text{ or } v_o) \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	



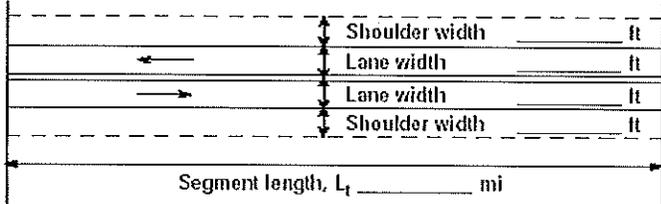
# Appendix 6

## Projected Two-Lane Highway 2035 – Low Condition

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	RP 0.6 to 20.0 NB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 Low

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

Input Data	
 <p>Shoulder width _____ ft</p> <p>Lane width _____ ft</p> <p>Lane width _____ ft</p> <p>Shoulder width _____ ft</p> <p>Segment length, <math>L_1</math> _____ mi</p> <p>Analysis direction vol., <math>V_d</math>      242veh/h</p> <p>Opposing direction vol., <math>V_o</math>      249veh/h</p> <p>Shoulder width ft                      8.0</p> <p>Lane Width ft                            12.0</p> <p>Segment Length mi                    19.4</p>	<div style="text-align: center;"> <input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway  <input type="checkbox"/> Class III highway                 </div> <p>Terrain    <input checked="" type="checkbox"/> Level    <input type="checkbox"/> Rolling</p> <p>Grade Length mi                      Up/down</p> <p>Peak-hour factor, PHF                0.81</p> <p>No-passing zone                        27%</p> <p>% Trucks and Buses , <math>P_T</math>            27%</p> <p>% Recreational vehicles, <math>P_R</math>        4%</p> <p>Access points mi                        5/mi</p> <div style="text-align: center;">  <p>Show North Arrow</p> </div>

Average Travel Speed		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.4	1.4
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.903	0.903
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	331	340
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS                      65.0 mi/h	
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)      0.0 mi/h	
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)                      1.3 mi/h	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)                      2.0 mi/h	Free-flow speed, FFS ( $FSS = BFFS - f_{LS} - f_A$ )                      63.8 mi/h	
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 56.5 mi/h	
	Percent free flow speed, PFFS    88.7 %	

Percent Time-Spent-Following		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.974	0.974
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	307	316
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-av_d^b})$	34.5	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	40.7	
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	54.6	

Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, v/c	0.22

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1535
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1655
Percent Free-Flow Speed $PF_{FS_d}$ (Equation 15-11 - Class III only)	88.7
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	298.8
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	13.66
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o</math> <math>\geq</math> 1,700 pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	RP 0.6 to RP 12.4 SB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 Low

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway     Class II highway

Class III highway

Terrain     Level     Rolling

Grade Length mi    Up/down

Peak-hour factor, PHF    0.78

No-passing zone    20%

% Trucks and Buses,  $P_T$     29%

% Recreational vehicles,  $P_R$     4%

Access points mi    7/mi

Show North Arrow

Analysis direction vol.,  $V_d$     249veh/h

Opposing direction vol.,  $V_o$     242veh/h

Shoulder width ft    8.0

Lane Width ft    12.0

Segment Length mi    11.8

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.4	1.4
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.896	0.896
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	356	346
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS    65.0 mi/h	
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)    0.0 mi/h	
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)    1.8 mi/h	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    1.7 mi/h	Free-flow speed, FFS ( $FSS = BFFS * f_{LS} * f_A$ )    63.3 mi/h	
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 56.1 mi/h	
	Percent free flow speed, PFFS    88.7 %	

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.972	0.972
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	328	319
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-a v_d^b})$	36.1	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	37.0	
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	54.9	

**Level of Service and Other Performance Measures**

Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, $v/c$	0.23

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1523
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1652
Percent Free-Flow Speed $PF_{FS_d}$ (Equation 15-11 - Class III only)	88.7
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	319.2
Effective width, $W_w$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	15.25
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_i(v_d \text{ or } v_o) \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

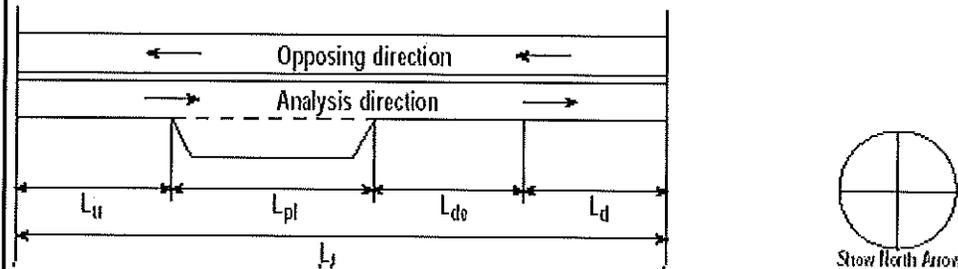
## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET WITH PASSING LANE WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway of Travel	MT 16
Agency or Company	DOWL HKM	From/To	RP 20.0 to Savage NB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 Low

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Class I highway  
  Class II highway  
  Class III highway



Shoulder width (ft)	8.0
Lane Width (ft)	12.0
Segment Length (mi)	11.5
Total length of analysis segment, $L_t$	11.5
Length of two-lane highway upstream of the passing lane, $L_u$	0.0
Length of passing lane including tapers, $L_{pl}$	1.9
Average travel speed, $ATS_d$ (from Directional Two-Lane Highway Segment Worksheet)	56.8
Percent time-spent-following, $PTSF_d$ (from Directional Two-Lane Highway Segment Worksheet)	53.0
Level of service <sup>1</sup> , $LOS_d$ (from Directional Two-Lane Highway Segment Worksheet)	C

**Average Travel Speed**

Length of the downstream highway segment within the effective length of passing lane for average travel speed, $L_{de}$ (Exhibit 15-23)	1.70
Length of two-lane highway downstream of effective length of the passing lane for avg travel speed, $L_d = L_t - (L_u + L_{pl} + L_{de})$	7.90
Adj. factor for the effect of passing lane on average speed, $f_{pl}$ (Exhibit 15-28)	1.10
Average travel speed including passing lane <sup>2</sup> , $ATS_{pl} = (ATS_d * L_t) / (L_u + L_d + (L_{pl}/f_{pl}) + (2L_{de}/(1+f_{pl}ATS_d)))$	58.1
Percent free flow speed including passing lane, $PFFS_{pl} = (ATS_{pl}/FFS)$	91.1

**Percent Time-Spent-Following**

Length of the downstream highway segment within the effective length of passing lane for percent time-spent-following, $L_{de}$ (Exhibit 15-23)	11.36
Length of two-lane highway downstream of effective length of the passing lane for percent-time-following, $L_d = L_t - (L_u + L_{pl} + L_{de})$	-1.76
Adj. factor for the effect of passing lane on percent time-spent-following, $f_{pl,PTSF}$ (Exhibit 15-26)	0.60

Percent time-spent-following including passing lane <sup>3</sup> , $PTSF_{pl}(\%)$ $PTSF_{pl} = PTSF_d [L_u + L_d + f_{pl, PTSF} L_{pl} + ((1 + f_{pl, PTSF}) / 2) L_{de}] / L_t$	39.3
<b>Level of Service and Other Performance Measures<sup>4</sup></b>	
Level of service including passing lane $LOS_{pl}$ (Exhibit 15-3)	B
Peak 15-min total travel time, $TT_{15}(\text{veh-h})$ $TT_{15} = VMT_{15} / ATS_{pl}$	14.8
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	298.8
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_t$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	13.66
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<ol style="list-style-type: none"> <li>1. If <math>LOS_d = F</math>, passing lane analysis cannot be performed.</li> <li>2. If <math>L_d &lt; 0</math>, use alternative Equation 15-18.</li> <li>3. If <math>L_d &lt; 0</math>, use alternative Equation 15-16.</li> <li>4. <i>v/c</i>, <math>VMT_{15}</math> and <math>VMT_{60}</math> are calculated on Directional Two-Lane Highway Segment Worksheet.</li> </ol>	

<b>DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET WITH PASSING LANE WORKSHEET</b>	
<b>General Information</b>	
Analyst	David Stoner
Agency or Company	DOWL HKM
Date Performed	4/17/2012
Analysis Time Period	Peak Hour
<b>Site Information</b>	
Highway of Travel	MT 16
From/To	RP 12.4 to RP 22.0 SB
Jurisdiction	Dawson/Richland County
Analysis Year	2035 Low
Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study	
<b>Input Data</b>	
<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway <input type="checkbox"/> Class III highway	
Shoulder width (ft)	8.0
Lane Width (ft)	12.0
Segment Length (mi)	9.6
Total length of analysis segment, $L_t$	9.6
Length of two-lane highway upstream of the passing lane, $L_u$	0.0
Length of passing lane including tapers, $L_{pl}$	1.9
Average travel speed, $ATS_d$ (from Directional Two-Lane Highway Segment Worksheet)	57.3
Percent time-spent-following, $PTSF_d$ (from Directional Two-Lane Highway Segment Worksheet)	52.7
Level of service <sup>1</sup> , $LOS_d$ (from Directional Two-Lane Highway Segment Worksheet)	C
<b>Average Travel Speed</b>	
Length of the downstream highway segment within the effective length of passing lane for average travel speed, $L_{de}$ (Exhibit 15-23)	1.70
Length of two-lane highway downstream of effective length of the passing lane for avg travel speed, $L_d = L_t - (L_u + L_{pl} + L_{de})$	6.00
Adj. factor for the effect of passing lane on average speed, $f_{pl}$ (Exhibit 15-28)	1.10
Average travel speed including passing lane <sup>2</sup> , $ATS_{pl} = (ATS_d * L_t) / (L_u + L_d + (L_{pl}/f_{pl}) + (2L_{de}/(1+f_{pl}/ATS_d)))$	58.9
Percent free flow speed including passing lane, $FFS_{pl} = (ATS_{pl}/FFS)$	91.3
<b>Percent Time-Spent-Following</b>	
Length of the downstream highway segment within the effective length of passing lane for percent time-spent-following, $L_{de}$ (Exhibit 15-23)	10.62
Length of two-lane highway downstream of effective length of the passing lane for percent-time-following, $L_d = L_t - (L_u + L_{pl} + L_{de})$	-2.92
Adj. factor for the effect of passing lane on percent time-spent-following, $f_{pl,PTSF}$ (Exhibit 15-26)	0.60

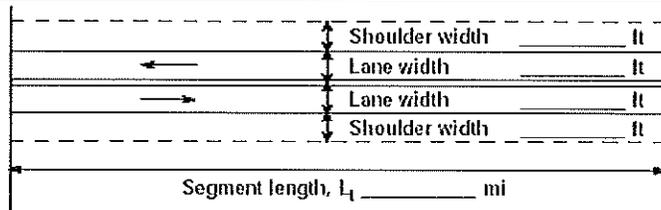
Percent time-spent-following including passing lane <sup>3</sup> , $PTSF_{pl}(\%)$ $PTSF_{pl} = PTSF_d [L_u + L_d + f_{pl, PTSF} L_{pl} + ((1 + f_{pl, PTSF})/2) L_{de}] / L_t$	37.7
<b>Level of Service and Other Performance Measures<sup>4</sup></b>	
Level of service including passing lane $LOS_{pl}$ (Exhibit 15-3)	B
Peak 15-min total travel time, $TT_{15}(\text{veh-h})$ $TT_{15} = VMT_{15}/ATS_{pl}$	13.0
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	319.2
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	15.25
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<ol style="list-style-type: none"> <li>1. If <math>LOS_d = F</math>, passing lane analysis cannot be performed.</li> <li>2. If <math>L_d &lt; 0</math>, use alternative Equation 15-18.</li> <li>3. If <math>L_d &lt; 0</math>, use alternative Equation 15-16.</li> <li>4. <math>v/c</math>, <math>VMT_{15}</math> and <math>VMT_{60}</math> are calculated on Directional Two-Lane Highway Segment Worksheet.</li> </ol>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	RP 22.0 to Savage SB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 Low

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**



Class I highway     Class II highway  
 Class III highway

Terrain  Level     Rolling  
 Up/down

Grade Length mi \_\_\_\_\_  
 Peak-hour factor, PHF 0.78  
 No-passing zone 22%  
 % Trucks and Buses, P<sub>T</sub> 29%  
 % Recreational vehicles, P<sub>R</sub> 4%  
 Access points mi 5/mi



Analysis direction vol., V<sub>d</sub> 249veh/h  
 Opposing direction vol., V<sub>o</sub> 242veh/h  
 Shoulder width ft 8.0  
 Lane Width ft 12.0  
 Segment Length mi 9.5

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, E <sub>T</sub> (Exhibit 15-11 or 15-12)	1.4	1.4
Passenger-car equivalents for RVs, E <sub>R</sub> (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, f <sub>HV,ATS</sub> =1/(1+P <sub>T</sub> (E <sub>T</sub> -1)+P <sub>R</sub> (E <sub>R</sub> -1))	0.896	0.896
Grade adjustment factor <sup>1</sup> , f <sub>g,ATS</sub> (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , v <sub>i</sub> (pc/h) v <sub>i</sub> =V <sub>i</sub> /(PHF* f <sub>g,ATS</sub> * f <sub>HV,ATS</sub> )	356	346
<b>Free-Flow Speed from Field Measurement</b>		<b>Estimated Free-Flow Speed</b>
Mean speed of sample <sup>3</sup> , S <sub>FM</sub>		Base free-flow speed <sup>4</sup> , BFFS 65.0 mi/h
Total demand flow rate, both directions, v		Adj. for lane and shoulder width <sup>4</sup> , f <sub>LS</sub> (Exhibit 15-7) 0.0 mi/h
Free-flow speed, FFS=S <sub>FM</sub> +0.00776(v/ f <sub>HV,ATS</sub> )		Adj. for access points <sup>4</sup> , f <sub>A</sub> (Exhibit 15-8) 1.3 mi/h
Adj. for no-passing zones, f <sub>np,ATS</sub> (Exhibit 15-15) 1.8 mi/h		Free-flow speed, FFS (FSS=BFFS-f <sub>LS</sub> -f <sub>A</sub> ) 63.8 mi/h
		Average travel speed, ATS <sub>d</sub> =FFS-0.00776(v <sub>d,ATS</sub> + v <sub>o,ATS</sub> )- f <sub>np,ATS</sub> 56.5 mi/h
		Percent free flow speed, PFFS 88.7 %

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, E <sub>T</sub> (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, E <sub>R</sub> (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, f <sub>HV</sub> =1/(1+P <sub>T</sub> (E <sub>T</sub> -1)+P <sub>R</sub> (E <sub>R</sub> -1))	0.972	0.972
Grade adjustment factor <sup>1</sup> , f <sub>g,PTSF</sub> (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , v <sub>i</sub> (pc/h) v <sub>i</sub> =V <sub>i</sub> /(PHF* f <sub>HV,PTSF</sub> * f <sub>g,PTSF</sub> )	328	319
Base percent time-spent-following <sup>4</sup> , BPTSF <sub>d</sub> (%)=100(1-e <sup>-av<sub>d</sub></sup> )		36.1
Adj. for no-passing zone, f <sub>np,PTSF</sub> (Exhibit 15-21)		37.9
Percent time-spent-following, PTSF <sub>d</sub> (%)=BPTSF <sub>d</sub> +f <sub>np,PTSF</sub> *(v <sub>d,PTSF</sub> /v <sub>d,PTSF</sub> + v <sub>o,PTSF</sub> )		55.3

**Level of Service and Other Performance Measures**

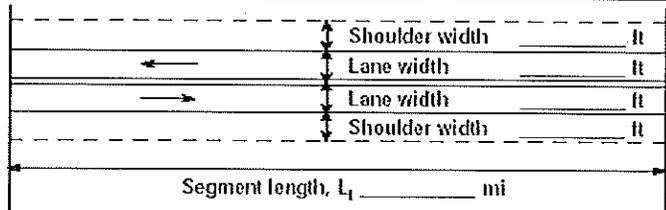
Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, v/c	0.23

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1523
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1652
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	88.7
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	319.2
Effective width, $W_w$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	15.25
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o</math> <math>\geq</math> 1,700 pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	Savage to Crane NB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

Input Data	
 <p>Shoulder width _____ ft</p> <p>Lane width _____ ft</p> <p>Lane width _____ ft</p> <p>Shoulder width _____ ft</p> <p>Segment length, <math>L_1</math> _____ mi</p> <p>Analysis direction vol., <math>V_d</math>      253veh/h</p> <p>Opposing direction vol., <math>V_o</math>      307veh/h</p> <p>Shoulder width ft                      8.0</p> <p>Lane Width ft                            12.0</p> <p>Segment Length mi                    10.0</p>	<div style="display: flex; justify-content: space-between;"> <span><input checked="" type="checkbox"/> Class I highway</span> <span><input type="checkbox"/> Class II highway</span> </div> <div style="display: flex; justify-content: space-between;"> <span><input type="checkbox"/> Class III highway</span> </div> <div style="display: flex; justify-content: space-between;"> <span>Terrain <input checked="" type="checkbox"/> Level</span> <span><input type="checkbox"/> Rolling</span> </div> <div style="display: flex; justify-content: space-between;"> <span>Grade Length mi</span> <span>Up/down</span> </div> <p>Peak-hour factor, PHF              0.87</p> <p>No-passing zone                      31%</p> <p>% Trucks and Buses, <math>P_T</math>          23 %</p> <p>% Recreational vehicles, <math>P_R</math>      4%</p> <p>Access points mi                      11/mi</p> <div style="text-align: center;">  <p>Show North Arrow</p> </div>

Average Travel Speed		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.4	1.3
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.916	0.935
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_f(pch) v_f = V_f / (PHF * f_{g,ATS} * f_{HV,ATS})$	317	377
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS                      69.0 mi/h	
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)      0.0 mi/h	
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)              2.8 mi/h	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)      2.1 mi/h	Free-flow speed, FFS ( $FFS = BFFS - f_{LS} - f_A$ )              66.3 mi/h	
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 58.8 mi/h	
	Percent free flow speed, PFFS                              88.7 %	

Percent Time-Spent-Following		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.978	0.978
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_f(pch) v_f = V_f / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	297	361
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(%) = 100(1 - e^{-av_d^b})$	33.3	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	39.9	
Percent time-spent-following, $PTSF_d(%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	51.3	

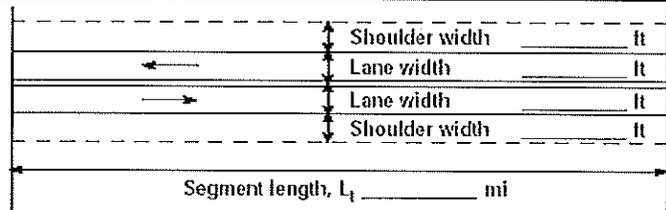
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, v/c	0.20

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1590
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1662
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	88.7
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	290.8
Effective width, $Wv$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_l$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	10.79
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o</math> <math>\geq</math> 1,700 pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	Savage to Crane SB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

Input Data	
 <p>Shoulder width _____ ft</p> <p>Lane width _____ ft</p> <p>Lane width _____ ft</p> <p>Shoulder width _____ ft</p> <p>Segment length, <math>L_1</math> _____ mi</p> <p>Analysis direction vol., <math>V_d</math>      307veh/h</p> <p>Opposing direction vol., <math>V_o</math>      253veh/h</p> <p>Shoulder width ft                      8.0</p> <p>Lane Width ft                            12.0</p> <p>Segment Length mi                    10.0</p>	<div style="text-align: center;"> <input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway  <input type="checkbox"/> Class III highway                 </div> <p>Terrain      <input checked="" type="checkbox"/> Level      <input type="checkbox"/> Rolling</p> <p>Grade Length mi      Up/down</p> <p>Peak-hour factor, PHF      0.84</p> <p>No-passing zone              19%</p> <p>% Trucks and Buses, <math>P_T</math>      25 %</p> <p>% Recreational vehicles, <math>P_R</math>      4%</p> <p>Access points mi                  11/mi</p> <div style="text-align: center;">                       Show North Arrow                 </div>

Average Travel Speed		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.3	1.4
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.930	0.909
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	393	331
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS      66.0 mi/h	
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)      0.0 mi/h	
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)      2.8 mi/h	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)      1.7 mi/h	Free-flow speed, FFS ( $FSS = BFFS * f_{LS} * f_A$ )      63.3 mi/h	
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 55.9 mi/h	
	Percent free flow speed, PFFS      88.4 %	

Percent Time-Spent-Following		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.976	0.976
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	375	309
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(%) = 100(1 - e^{-a v_d^b})$	38.7	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	33.9	
Percent time-spent-following, $PTSF_d(%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	57.3	

Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, $v/c$	0.25

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1545
Capacity, $C_{d,PISF}$ (Equation 15-13) pc/h	1659
Percent Free-Flow Speed $PF_{FS_d}$ (Equation 15-11 - Class III only)	88.4
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	365.5
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	12.29
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o</math> <math>\geq</math> 1,700 pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	Crane to Sidney NB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway     Class II highway     Class III highway

Terrain     Level     Rolling

Grade Length mi    Up/down

Peak-hour factor, PHF    0.80

No-passing zone    24%

% Trucks and Buses,  $P_T$     19%

% Recreational vehicles,  $P_R$     4%

Access points mi    12/mi

Show North Arrow

Analysis direction vol.,  $V_d$     271veh/h

Opposing direction vol.,  $V_o$     416veh/h

Shoulder width ft    8.0

Lane Width ft    12.0

Segment Length mi    8.9

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.4	1.2
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.929	0.963
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{g,ATS} * f_{HV,ATS})$	365	540
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS    65.0 mi/h	
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)    0.0 mi/h	
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)    3.0 mi/h	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    1.4 mi/h	Free-flow speed, FFS ( $FSS = BFFS - f_{LS} - f_A$ )    62.0 mi/h	
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 53.6 mi/h	
	Percent free flow speed, PFFS    86.5 %	

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.981	1.000
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	345	520
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-av_d^b})$	41.1	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	27.9	
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	52.2	

**Level of Service and Other Performance Measures**

Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, v/c	0.22

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1637
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1700
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	86.5
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	338.8
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	8.34
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o</math> <math>\geq</math> 1,700 pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	Crane to Sidney SB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low
Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
Input Data			
		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway <input type="checkbox"/> Class III highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Grade Length    mi    Up/down Peak-hour factor, PHF    0.87 No-passing zone    22% % Trucks and Buses, P <sub>T</sub> 19% % Recreational vehicles, P <sub>R</sub> 4% Access points    mi    12/mi	
Analysis direction vol., V <sub>d</sub>	416veh/h	Show North Arrow	
Opposing direction vol., V <sub>o</sub>	271veh/h		
Shoulder width ft	8.0		
Lane Width ft	12.0		
Segment Length mi	8.9		
Average Travel Speed			
		Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, E <sub>T</sub> (Exhibit 15-11 or 15-12)		1.2	1.4
Passenger-car equivalents for RVs, E <sub>R</sub> (Exhibit 15-11 or 15-13)		1.0	1.0
Heavy-vehicle adjustment factor, f <sub>HV,ATS</sub> = 1 / (1 + P <sub>T</sub> (E <sub>T</sub> -1) + P <sub>R</sub> (E <sub>R</sub> -1))		0.963	0.929
Grade adjustment factor <sup>1</sup> , f <sub>g,ATS</sub> (Exhibit 15-9)		1.00	1.00
Demand flow rate <sup>2</sup> , v <sub>f</sub> (pc/h) v <sub>f</sub> = V <sub>f</sub> / (PHF * f <sub>g,ATS</sub> * f <sub>HV,ATS</sub> )		497	335
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed		
Mean speed of sample <sup>3</sup> , S <sub>FM</sub>		Base free-flow speed <sup>4</sup> , BFFS    69.0 mi/h	
Total demand flow rate, both directions, v		Adj. for lane and shoulder width <sup>4</sup> , f <sub>LS</sub> (Exhibit 15-7)    0.0 mi/h	
Free-flow speed, FFS = S <sub>FM</sub> + 0.00776(v / f <sub>HV,ATS</sub> )		Adj. for access points <sup>4</sup> , f <sub>A</sub> (Exhibit 15-8)    3.0 mi/h	
Adj. for no-passing zones, f <sub>np,ATS</sub> (Exhibit 15-15)    1.9 mi/h		Free-flow speed, FFS (FSS = BFFS * f <sub>LS</sub> * f <sub>A</sub> )    66.0 mi/h	
		Average travel speed, ATS <sub>d</sub> = FFS - 0.00776(v <sub>d,ATS</sub> + v <sub>o,ATS</sub> ) - f <sub>np,ATS</sub> 57.7 mi/h	
		Percent free flow speed, PFFS    87.4 %	
Percent Time-Spent-Following			
		Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, E <sub>T</sub> (Exhibit 15-18 or 15-19)		1.0	1.1
Passenger-car equivalents for RVs, E <sub>R</sub> (Exhibit 15-18 or 15-19)		1.0	1.0
Heavy-vehicle adjustment factor, f <sub>HV</sub> = 1 / (1 + P <sub>T</sub> (E <sub>T</sub> -1) + P <sub>R</sub> (E <sub>R</sub> -1))		1.000	0.981
Grade adjustment factor <sup>1</sup> , f <sub>g,PTSF</sub> (Exhibit 15-16 or Ex 15-17)		1.00	1.00
Directional flow rate <sup>2</sup> , v <sub>f</sub> (pc/h) v <sub>f</sub> = V <sub>f</sub> / (PHF * f <sub>HV,PTSF</sub> * f <sub>g,PTSF</sub> )		478	317
Base percent time-spent-following <sup>4</sup> , BPTSF <sub>d</sub> (%) = 100(1 - e <sup>-av<sub>d</sub></sup> )		47.4	
Adj. for no-passing zone, f <sub>np,PTSF</sub> (Exhibit 15-21)		28.7	
Percent time-spent-following, PTSF <sub>d</sub> (%) = BPTSF <sub>d</sub> + f <sub>np,PTSF</sub> * (v <sub>d,PTSF</sub> / v <sub>d,PTSF</sub> + v <sub>o,PTSF</sub> )		64.7	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 15-3)		C	
Volume to capacity ratio, v/c		0.31	

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1579
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1668
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	87.4
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	478.2
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	8.51
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_1(v_d \text{ or } v_o) \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 200
Agency or Company	DOWL HKM	From/To	Sidney to Fairview EB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway     Class II highway

Class III highway

Terrain     Level     Rolling

Grade Length mi    Up/down

Peak-hour factor, PHF    0.83

No-passing zone    17%

% Trucks and Buses,  $P_T$     17%

% Recreational vehicles,  $P_R$     4%

Access points mi    16/mi

Analysis direction vol., $V_d$	529veh/h		
Opposing direction vol., $V_o$	523veh/h		
Shoulder width ft	8.0		
Lane Width ft	12.0		
Segment Length mi	9.9		

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.983	0.983
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	648	641
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS    69.0 mi/h	
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)    0.0 mi/h	
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)    4.0 mi/h	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    1.3 mi/h	Free-flow speed, FFS ( $FSS = BFFS * f_{LS} * f_A$ )    65.0 mi/h	
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) * f_{np,ATS}$ 53.7 mi/h	
	Percent free flow speed, PFFS    82.7 %	

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.0	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	1.000	1.000
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	637	630
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(%) = 100(1 - e^{-a v_d^b})$	60.4	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	21.6	
Percent time-spent-following, $PTSF_d(%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	71.3	

**Level of Service and Other Performance Measures**

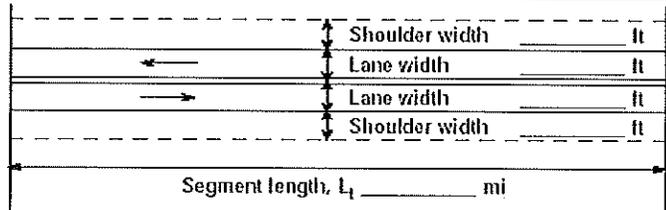
Level of service, LOS (Exhibit 15-3)	D
Volume to capacity ratio, v/c	0.39

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1671
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1700
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	82.7
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	637.3
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	7.52
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_1(v_d \text{ or } v_o) \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 200
Agency or Company	DOWL HKM	From/To	Sidney to Fairview WB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

Input Data	
 <p>Shoulder width _____ ft</p> <p>Lane width _____ ft</p> <p>Lane width _____ ft</p> <p>Shoulder width _____ ft</p> <p>Segment length, <math>L_1</math> _____ mi</p> <p>Analysis direction vol., <math>V_d</math>      523veh/h</p> <p>Opposing direction vol., <math>V_o</math>      529veh/h</p> <p>Shoulder width ft                      8.0</p> <p>Lane Width ft                            12.0</p> <p>Segment Length mi                    9.9</p>	<div style="text-align: center;">  <p>Show North Arrow</p> </div> <p><input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway</p> <p><input type="checkbox"/> Class III highway</p> <p>Terrain    <input checked="" type="checkbox"/> Level    <input type="checkbox"/> Rolling</p> <p>Grade Length mi                      Up/down</p> <p>Peak-hour factor, PHF                0.86</p> <p>No-passing zone                        15%</p> <p>% Trucks and Buses, <math>P_T</math>            25 %</p> <p>% Recreational vehicles, <math>P_R</math>        4%</p> <p>Access points mi                        16/mi</p>

Average Travel Speed		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.976	0.976
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{g,ATS} * f_{HV,ATS})$	623	630
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS                                      66.0 mi/h	
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)      0.0 mi/h	
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)                    4.0 mi/h	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)                      1.1 mi/h	Free-flow speed, FFS ( $FFS = BFFS - f_{LS} - f_A$ )                    62.0 mi/h	
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 51.1 mi/h	
	Percent free flow speed, PFFS                                      82.5 %	

Percent Time-Spent-Following		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.0	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	1.000	1.000
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	608	615
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-av_d^b})$	58.6	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	21.4	
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	69.2	

Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 15-3)	D
Volume to capacity ratio, v/c	0.38

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1659
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1700
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	82.5
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	608.1
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	12.55
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o \geq 1,700</math> pc/h, terminate analysis—the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	



# Appendix 6

## Projected Two-Lane Highway 2035 – High Condition

## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	RP 0.6 to 20.0 NB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 High

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway     Class II highway

Class III highway

Terrain     Level     Rolling

Grade Length mi    Up/down

Peak-hour factor, PHF    0.81

No-passing zone    27%

% Trucks and Buses,  $P_T$     27%

% Recreational vehicles,  $P_R$     4%

Access points mi    5/mi

Show North Arrow

Analysis direction vol.,  $V_d$     321veh/h

Opposing direction vol.,  $V_o$     331veh/h

Shoulder width ft    8.0

Lane Width ft    12.0

Segment Length mi    19.4

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.3	1.3
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.925	0.925
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{g,ATS} * f_{HV,ATS})$	428	442

Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS	65.0 mi/h
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)	0.0 mi/h
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)	1.3 mi/h
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    1.7 mi/h	Free-flow speed, FFS ( $FSS = BFFS * f_{LS} * f_A$ )	63.8 mi/h
	Average travel speed, $ATS_d = FFS - 0.00776(v_d,ATS + v_o,ATS) * f_{np,ATS}$	55.3 mi/h
	Percent free flow speed, PFFS	86.7 %

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.974	1.000
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	407	409

Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-av_d^b})$	42.7
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	35.3
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_d,PTSF / v_d,PTSF + v_o,PTSF)$	60.3

**Level of Service and Other Performance Measures**

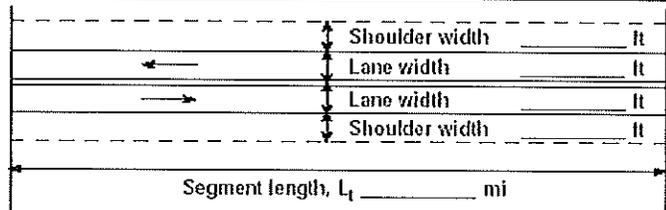
Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, v/c	0.27

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1573
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1700
Percent Free-Flow Speed $PF_{FS_d}$ (Equation 15-11 - Class III only)	86.7
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	396.3
Effective width, $W_w$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	13.81
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_1(v_d \text{ or } v_o) \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	RP 0.6 to RP 12.4 SB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 High

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

Input Data	
 <p style="margin-left: 20px;">Shoulder width _____ ft</p> <p style="margin-left: 20px;">Lane width _____ ft</p> <p style="margin-left: 20px;">Lane width _____ ft</p> <p style="margin-left: 20px;">Shoulder width _____ ft</p> <p style="margin-left: 20px;">Segment length, <math>L_1</math> _____ mi</p>	<div style="display: flex; justify-content: space-between;"> <span><input checked="" type="checkbox"/> Class I highway</span> <span><input type="checkbox"/> Class II highway</span> </div> <div style="display: flex; justify-content: space-between;"> <span><input type="checkbox"/> Class III highway</span> </div> <div style="display: flex; justify-content: space-between;"> <span>Terrain <input checked="" type="checkbox"/> Level</span> <span><input type="checkbox"/> Rolling</span> </div> <p>Grade Length mi Up/down _____</p> <p>Peak-hour factor, PHF 0.78</p> <p>No-passing zone 20%</p> <p>% Trucks and Buses, <math>P_T</math> 29%</p> <p>% Recreational vehicles, <math>P_R</math> 4%</p> <p>Access points mi 7/mi</p> <div style="text-align: center;">  <p>Show North Arrow</p> </div>
<p>Analysis direction vol., <math>V_d</math> 331veh/h</p> <p>Opposing direction vol., <math>V_o</math> 321veh/h</p> <p>Shoulder width ft 8.0</p> <p>Lane Width ft 12.0</p> <p>Segment Length mi 11.8</p>	

Average Travel Speed		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.3	1.3
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.920	0.920
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	461	447
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed
Mean speed of sample <sup>3</sup> , $S_{FM}$		Base free-flow speed <sup>4</sup> , BFFS 65.0 mi/h
Total demand flow rate, both directions, $v$		Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7) 0.0 mi/h
Free-flow speed, $FSS = S_{FM} + 0.00776(v / f_{HV,ATS})$		Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8) 1.8 mi/h
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15) 1.5 mi/h		Free-flow speed, FFS ( $FSS = BFFS * f_{LS} * f_A$ ) 63.3 mi/h
		Average travel speed, $ATS_d = FFS * 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 54.7 mi/h
		Percent free flow speed, PFFS 86.5 %

Percent Time-Spent-Following		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.0	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	1.000	1.000
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	424	412
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(%) = 100(1 - e^{-av_d^b})$		45.2
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)		32.6
Percent time-spent-following, $PTSF_d(%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$		61.7

Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, v/c	0.29

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1564
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1700
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	86.5
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	424.4
Effective width, $W_w$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	15.39
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_1(v_d \text{ or } v_o) \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

<b>DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET WITH PASSING LANE WORKSHEET</b>			
<b>General Information</b>		<b>Site Information</b>	
Analyst	David Stoner	Highway of Travel	MT 16
Agency or Company	DOWL HKM	From/To	RP 20.0 to Savage NB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 High
Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<b>Input Data</b>			
<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway <input type="checkbox"/> Class III highway			
Shoulder width (ft)		8.0	
Lane Width (ft)		12.0	
Segment Length (mi)		11.5	
Total length of analysis segment, $L_t$		11.5	
Length of two-lane highway upstream of the passing lane, $L_u$		0.0	
Length of passing lane including tapers, $L_{pl}$		1.9	
Average travel speed, $ATS_d$ (from Directional Two-Lane Highway Segment Worksheet)		55.5	
Percent time-spent-following, $PTSF_d$ (from Directional Two-Lane Highway Segment Worksheet)		59.2	
Level of service <sup>1</sup> , $LOS_d$ (from Directional Two-Lane Highway Segment Worksheet)		C	
<b>Average Travel Speed</b>			
Length of the downstream highway segment within the effective length of passing lane for average travel speed, $L_{de}$ (Exhibit 15-23)		1.70	
Length of two-lane highway downstream of effective length of the passing lane for avg travel speed, $L_d = L_t - (L_u + L_{pl} + L_{de})$		7.90	
Adj. factor for the effect of passing lane on average speed, $f_{pl}$ (Exhibit 15-28)		1.10	
Average travel speed including passing lane <sup>2</sup> , $ATS_{pl} = (ATS_d + L_t) / (L_u + L_d + (L_{pl}/f_{pl}) + (2L_{de}/(1+f_{pl}ATS_d)))$		56.7	
Percent free flow speed including passing lane, $FFFS_{pl} = (ATS_{pl} / FFS)$		89.0	
<b>Percent Time-Spent-Following</b>			
Length of the downstream highway segment within the effective length of passing lane for percent time-spent-following, $L_{de}$ (Exhibit 15-23)		8.04	
Length of two-lane highway downstream of effective length of the passing lane for percent-time-following, $L_d = L_t - (L_u + L_{pl} + L_{de})$		1.56	
Adj. factor for the effect of passing lane on percent time-spent-following, $f_{pl,PTSF}$ (Exhibit 15-26)		0.61	

Percent time-spent-following including passing lane <sup>3</sup> , PTSF <sub>pl</sub> (%) $PTSF_{pl} = PTSF_d [ L_u + L_d + f_{pl, PTSF} L_{pl} + ((1 + f_{pl, PTSF}) / 2) L_{de} ] / L_t$	47.3
<b>Level of Service and Other Performance Measures<sup>4</sup></b>	
Level of service including passing lane LOS <sub>pl</sub> (Exhibit 15-3)	B
Peak 15-min total travel time, TT <sub>15</sub> (veh-h)    TT <sub>15</sub> = VMT <sub>15</sub> /ATS <sub>pl</sub>	20.1
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, v <sub>OL</sub> (Eq. 15-24) veh/h	396.3
Effective width, W <sub>v</sub> (Eq. 15-29) ft	28.00
Effective speed factor, S <sub>f</sub> (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	13.81
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
1. If LOS <sub>d</sub> =F, passing lane analysis cannot be performed. 2. If L <sub>d</sub> < 0, use alternative Equation 15-18. 3. If L <sub>q</sub> < 0, use alternative Equation 15-16. 4. v/c, VMT <sub>15</sub> and VMT <sub>60</sub> are calculated on Directional Two-Lane Highway Segment Worksheet.	

<b>DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET WITH PASSING LANE WORKSHEET</b>	
<b>General Information</b>	
Analyst	David Stoner
Agency or Company	DOWL HKM
Date Performed	4/17/2012
Analysis Time Period	Peak Hour
<b>Site Information</b>	
Highway of Travel	MT 16
From/To	RP 12.4 to RP 22.0 SB
Jurisdiction	Dawson/Richland County
Analysis Year	2035 High
Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study	
<b>Input Data</b>	
<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway <input type="checkbox"/> Class III highway	
Shoulder width (ft)	8.0
Lane Width (ft)	12.0
Segment Length (mi)	9.6
Total length of analysis segment, $L_t$	9.6
Length of two-lane highway upstream of the passing lane, $L_u$	0.0
Length of passing lane including tapers, $L_{pl}$	1.9
Average travel speed, $ATS_d$ (from Directional Two-Lane Highway Segment Worksheet)	55.9
Percent time-spent-following, $PTSF_d$ (from Directional Two-Lane Highway Segment Worksheet)	60.0
Level of service <sup>1</sup> , $LOS_d$ (from Directional Two-Lane Highway Segment Worksheet)	C
<b>Average Travel Speed</b>	
Length of the downstream highway segment within the effective length of passing lane for average travel speed, $L_{de}$ (Exhibit 15-23)	1.70
Length of two-lane highway downstream of effective length of the passing lane for avg travel speed, $L_d = L_t - (L_u + L_{pl} + L_{de})$	6.00
Adj. factor for the effect of passing lane on average speed, $f_{pl}$ (Exhibit 15-28)	1.10
Average travel speed including passing lane <sup>2</sup> , $ATS_{pl} = (ATS_d * L_t) / (L_u + L_d + (L_{pl}/f_{pl}) + (2L_{de}/(1+f_{pl,ATS})))$	57.4
Percent free flow speed including passing lane, $PFFS_{pl} = (ATS_{pl} / FFS)$	89.1
<b>Percent Time-Spent-Following</b>	
Length of the downstream highway segment within the effective length of passing lane for percent time-spent-following, $L_{de}$ (Exhibit 15-23)	7.91
Length of two-lane highway downstream of effective length of the passing lane for percent-time-following, $L_d = L_t - (L_u + L_{pl} + L_{de})$	-0.21
Adj. factor for the effect of passing lane on percent time-spent-following, $f_{pl,PTSF}$ (Exhibit 15-26)	0.61

Percent time-spent-following including passing lane <sup>3</sup> , $PTSF_{pl}(\%)$ $PTSF_{pl} = PTSF_d [L_u + L_d + f_{pl, PTSF} L_{pl} + ((1 + f_{pl, PTSF}) / 2) L_{de}] / L_t$	45.7
<b>Level of Service and Other Performance Measures<sup>4</sup></b>	
Level of service including passing lane $LOS_{pl}$ (Exhibit 15-3)	B
Peak 15-min total travel time, $TT_{15}(\text{veh-h})$ $TT_{15} = VMT_{15} / ATS_{pl}$	17.7
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	424.4
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_t$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	15.39
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
1. If $LOS_d = F$ , passing lane analysis cannot be performed. 2. If $L_d < 0$ , use alternative Equation 15-18. 3. If $L_d < 0$ , use alternative Equation 15-16. 4. $v/c$ , $VMT_{15}$ and $VMT_{60}$ are calculated on Directional Two-Lane Highway Segment Worksheet.	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	RP 22.0 to Savage SB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 High

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway     Class II highway  
 Class III highway

Terrain     Level     Rolling  
 Grade Length mi    Up/down

Peak-hour factor, PHF    0.78  
 No-passing zone    22%  
 % Trucks and Buses,  $P_T$     29%  
 % Recreational vehicles,  $P_R$     4%  
 Access points mi    5/mi

Analysis direction vol., $V_d$	321veh/h
Opposing direction vol., $V_o$	331veh/h
Shoulder width ft	8.0
Lane Width ft	12.0
Segment Length mi	9.5

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.3	1.3
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.920	0.920
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	447	461
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS    65.0 mi/h	
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)    0.0 mi/h	
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)    1.3 mi/h	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    1.5 mi/h	Free-flow speed, FFS (FSS=BFFS- $f_{LS}$ - $f_A$ )    63.8 mi/h	
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 55.2 mi/h	
	Percent free flow speed, PFFS    86.5 %	

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.0	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	1.000	1.000
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	412	424
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-a v_d^b})$	43.7	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	33.2	
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	60.1	

**Level of Service and Other Performance Measures**

Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, v/c	0.29

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1564
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1700
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	86.5
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	411.5
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	15.38
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	Savage to Crane NB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Shoulder width \_\_\_\_\_ ft  
Lane width \_\_\_\_\_ ft  
Lane width \_\_\_\_\_ ft  
Shoulder width \_\_\_\_\_ ft

Segment length,  $L_t$  \_\_\_\_\_ mi

Class I highway     Class II highway

Class III highway

Terrain     Level     Rolling

Grade Length mi    Up/down

Peak-hour factor, PHF    0.87

No-passing zone    31%

% Trucks and Buses,  $P_T$     23%

% Recreational vehicles,  $P_R$     4%

Access points mi    11/mi

Show North Arrow

Analysis direction vol.,  $V_d$     336veh/h

Opposing direction vol.,  $V_o$     407veh/h

Shoulder width ft    8.0

Lane Width ft    12.0

Segment Length mi    10.0

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.3	1.2
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.935	0.956
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{g,ATS} * f_{HV,ATS})$	413	489
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS    69.0 mi/h	
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)    0.0 mi/h	
Free-flow speed, $FFS = S_{FM} + 0.00776(v f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)    2.8 mi/h	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    1.7 mi/h	Free-flow speed, FFS ( $FSS = BFFS - f_{LS} - f_A$ )    66.3 mi/h	
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 57.5 mi/h	
	Percent free flow speed, PFFS    86.8 %	

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.1	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.978	1.000
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	395	468
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-a v_d^b})$	44.1	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	33.0	
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	59.2	

**Level of Service and Other Performance Measures**

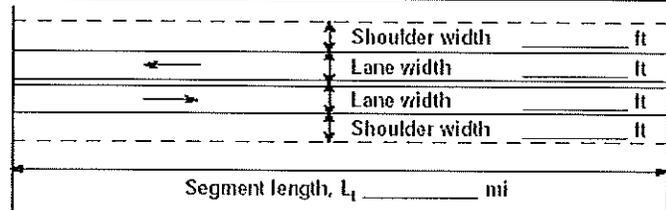
Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, $v/c$	0.25

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1625
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1700
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	86.8
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	386.2
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	10.93
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	Savage to Crane SB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

Input Data	
 <p>Shoulder width _____ ft</p> <p>Lane width _____ ft</p> <p>Lane width _____ ft</p> <p>Shoulder width _____ ft</p> <p>Segment length, <math>L_1</math> _____ mi</p> <p>Analysis direction vol., <math>V_d</math>      407veh/h</p> <p>Opposing direction vol., <math>V_o</math>      336veh/h</p> <p>Shoulder width ft                      8.0</p> <p>Lane Width ft                            12.0</p> <p>Segment Length mi                    10.0</p>	<div style="text-align: center;">  <p>Store North Arrow</p> </div> <p><input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway</p> <p><input type="checkbox"/> Class III highway</p> <p>Terrain    <input checked="" type="checkbox"/> Level    <input type="checkbox"/> Rolling</p> <p>Grade Length mi    Up/down</p> <p>Peak-hour factor, PHF      0.84</p> <p>No-passing zone              19%</p> <p>% Trucks and Buses , <math>P_T</math>      25 %</p> <p>% Recreational vehicles, <math>P_R</math>    4%</p> <p>Access points mi                11/mi</p>

Average Travel Speed		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.2	1.3
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.952	0.930
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{g,ATS} * f_{HV,ATS})$	509	430
<b>Free-Flow Speed from Field Measurement</b>		<b>Estimated Free-Flow Speed</b>
Mean speed of sample <sup>3</sup> , $S_{FM}$		Base free-flow speed <sup>4</sup> , BFFS      66.0 mi/h
Total demand flow rate, both directions, $v$		Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)      0.0 mi/h
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$		Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)      2.8 mi/h
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)      1.5 mi/h		Free-flow speed, FFS ( $FSS = BFFS - f_{LS} - f_A$ )      63.3 mi/h
		Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 54.5 mi/h
		Percent free flow speed, PFFS      86.1 %

Percent Time-Spent-Following		
	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.0	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	1.000	0.976
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	485	410
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-av_d^b})$		49.3
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)		28.5
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$		64.7

Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, v/c	0.32

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1581
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1700
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	86.1
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	484.5
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	12.44
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 16
Agency or Company	DOWL HKM	From/To	Crane to Sidney NB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Shoulder width \_\_\_\_\_ ft  
Lane width \_\_\_\_\_ ft  
Lane width \_\_\_\_\_ ft  
Shoulder width \_\_\_\_\_ ft

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway     Class II highway  
 Class III highway

Terrain     Level     Rolling

Grade Length mi    Up/down

Peak-hour factor, PHF    0.80

No-passing zone    24%

% Trucks and Buses,  $P_T$     19%

% Recreational vehicles,  $P_R$     4%

Access points mi    12/mi

Show North Arrow

Analysis direction vol.,  $V_d$     360veh/h

Opposing direction vol.,  $V_o$     552veh/h

Shoulder width ft    8.0

Lane Width ft    12.0

Segment Length mi    8.9

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.3	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.946	0.981
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{g,ATS} * f_{HV,ATS})$	476	703
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS    65.0 mi/h	
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)    0.0 mi/h	
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)    3.0 mi/h	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    1.0 mi/h	Free-flow speed, FFS ( $FSS = BFFS - f_{LS} - f_A$ )    62.0 mi/h	
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 51.9 mi/h	
	Percent free flow speed, PFFS    83.7 %	

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.0	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	1.000	1.000
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	450	690
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-av_d^b})$	50.2	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	23.5	
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	59.5	

**Level of Service and Other Performance Measures**

Level of service, LOS (Exhibit 15-3)	C
Volume to capacity ratio, $v/c$	0.29

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1668
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1700
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	83.7
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	450.0
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	8.48
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst <i>David Stoner</i>	<i>DOWL HKM</i>	Highway / Direction of Travel <i>MT 16</i>	<i>Crane to Sidney SB</i>
Agency or Company <i>DOWL HKM</i>	Date Performed <i>4/17/2012</i>	From/To <i>Crane to Sidney SB</i>	Jurisdiction <i>Dawson/Richland County</i>
Date Performed <i>4/17/2012</i>	Analysis Time Period <i>Peak Hour</i>	Jurisdiction <i>Dawson/Richland County</i>	Analysis Year <i>2035 - High</i>

Project Description: *MT 16 / MT 200 Glendive to Fairview Corridor Planning Study*

**Input Data**

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway     Class II highway

Class III highway

Terrain     Level     Rolling

Grade Length mi    Up/down

Peak-hour factor, PHF    *0.87*

No-passing zone    *22%*

% Trucks and Buses,  $P_T$     *19%*

% Recreational vehicles,  $P_R$     *4%*

Access points mi    *12/mi*

Show North Arrow

Analysis direction vol.,  $V_d$     *552veh/h*

Opposing direction vol.,  $V_o$     *360veh/h*

Shoulder width ft    *8.0*

Lane Width ft    *12.0*

Segment Length mi    *8.9*

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	<i>1.1</i>	<i>1.3</i>
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	<i>1.0</i>	<i>1.0</i>
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	<i>0.981</i>	<i>0.946</i>
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	<i>1.00</i>	<i>1.00</i>
Demand flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{g,ATS} * f_{HV,ATS})$	<i>647</i>	<i>437</i>
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$	<i>69.0 mi/h</i>	
Total demand flow rate, both directions, $v$	<i>0.0 mi/h</i>	
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	<i>3.0 mi/h</i>	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15) <i>1.6 mi/h</i>	<i>66.0 mi/h</i>	
	<i>56.0 mi/h</i>	
	<i>84.8 %</i>	

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	<i>1.0</i>	<i>1.0</i>
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	<i>1.0</i>	<i>1.0</i>
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	<i>1.000</i>	<i>1.000</i>
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	<i>1.00</i>	<i>1.00</i>
Directional flow rate <sup>2</sup> , $v_f$ (pc/h) $v_f = V_f / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	<i>634</i>	<i>414</i>
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-av_d^b})$	<i>57.9</i>	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	<i>24.6</i>	
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	<i>72.8</i>	

**Level of Service and Other Performance Measures**

Level of service, LOS (Exhibit 15-3)	<i>D</i>
Volume to capacity ratio, v/c	<i>0.40</i>

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1608
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1700
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	84.8
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	634.5
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_l$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	8.66
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o</math> <math>\geq</math> 1,700 pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 200
Agency or Company	DOWL HKM	From/To	Sidney to Fairview EB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Shoulder width \_\_\_\_\_ ft  
Lane width \_\_\_\_\_ ft  
Lane width \_\_\_\_\_ ft  
Shoulder width \_\_\_\_\_ ft

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway     Class II highway

Class III highway

Terrain  Level     Rolling

Grade Length mi    Up/down

Peak-hour factor, PHF    0.83

No-passing zone    17%

% Trucks and Buses,  $P_T$     17%

% Recreational vehicles,  $P_R$     4%

Access points mi    16/mi

Analysis direction vol., $V_d$	661veh/h
Opposing direction vol., $V_o$	654veh/h
Shoulder width ft	8.0
Lane Width ft	12.0
Segment Length mi	9.9

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.983	0.983
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	810	802
<b>Free-Flow Speed from Field Measurement</b>		<b>Estimated Free-Flow Speed</b>
Mean speed of sample <sup>3</sup> , $S_{FM}$		Base free-flow speed <sup>4</sup> , BFFS    69.0 mi/h
Total demand flow rate, both directions, $v$		Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)    0.0 mi/h
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$		Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)    4.0 mi/h
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    0.7 mi/h		Free-flow speed, FFS ( $FSS = BFFS - f_{LS} - f_A$ )    65.0 mi/h
		Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) - f_{np,ATS}$ 51.8 mi/h
		Percent free flow speed, PFFS    79.7 %

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.0	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	1.000	1.000
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	796	788
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-a v_d^b})$		68.8
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)		17.1
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$		77.4

**Level of Service and Other Performance Measures**

Level of service, LOS (Exhibit 15-3)	D
Volume to capacity ratio, $v/c$	0.48

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1671
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1700
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	79.7
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	796.4
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	7.63
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	

### DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	David Stoner	Highway / Direction of Travel	MT 200
Agency or Company	DOWL HKM	From/To	Sidney to Fairview WB
Date Performed	4/17/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High

Project Description: MT 16 / MT 200 Glendive to Fairview Corridor Planning Study

**Input Data**

Shoulder width \_\_\_\_\_ ft  
Lane width \_\_\_\_\_ ft  
Lane width \_\_\_\_\_ ft  
Shoulder width \_\_\_\_\_ ft

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway     Class II highway

Class III highway

Terrain  Level     Rolling

Grade Length mi    Up/down

Peak-hour factor, PHF    0.86

No-passing zone    15%

% Trucks and Buses,  $P_T$     25 %

% Recreational vehicles,  $P_R$     4%

Access points mi    16/mi

Show North Arrow

Analysis direction vol., $V_d$	654veh/h
Opposing direction vol., $V_o$	661veh/h
Shoulder width ft	8.0
Lane Width ft	12.0
Segment Length mi	9.9

**Average Travel Speed**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-11 or 15-12)	1.1	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-11 or 15-13)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV,ATS} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	0.976	0.976
Grade adjustment factor <sup>1</sup> , $f_{g,ATS}$ (Exhibit 15-9)	1.00	1.00
Demand flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{g,ATS} * f_{HV,ATS})$	779	788
<b>Free-Flow Speed from Field Measurement</b>	<b>Estimated Free-Flow Speed</b>	
Mean speed of sample <sup>3</sup> , $S_{FM}$	Base free-flow speed <sup>4</sup> , BFFS    66.0 mi/h	
Total demand flow rate, both directions, $v$	Adj. for lane and shoulder width <sup>4</sup> , $f_{LS}$ (Exhibit 15-7)    0.0 mi/h	
Free-flow speed, $FFS = S_{FM} + 0.00776(v / f_{HV,ATS})$	Adj. for access points <sup>4</sup> , $f_A$ (Exhibit 15-8)    4.0 mi/h	
Adj. for no-passing zones, $f_{np,ATS}$ (Exhibit 15-15)    0.7 mi/h	Free-flow speed, FFS ( $FFS = BFFS * f_{LS} * f_A$ )    62.0 mi/h	
	Average travel speed, $ATS_d = FFS - 0.00776(v_{d,ATS} + v_{o,ATS}) * f_{np,ATS}$ 49.2 mi/h	
	Percent free flow speed, PFFS    79.3 %	

**Percent Time-Spent-Following**

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 15-18 or 15-19)	1.0	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 15-18 or 15-19)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$	1.000	1.000
Grade adjustment factor <sup>1</sup> , $f_{g,PTSF}$ (Exhibit 15-16 or Ex 15-17)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i = V_i / (PHF * f_{HV,PTSF} * f_{g,PTSF})$	760	769
Base percent time-spent-following <sup>4</sup> , $BPTSF_d(\%) = 100(1 - e^{-av_d^b})$	67.5	
Adj. for no-passing zone, $f_{np,PTSF}$ (Exhibit 15-21)	17.0	
Percent time-spent-following, $PTSF_d(\%) = BPTSF_d + f_{np,PTSF} * (v_{d,PTSF} / v_{d,PTSF} + v_{o,PTSF})$	75.9	

**Level of Service and Other Performance Measures**

Level of service, LOS (Exhibit 15-3)	D
Volume to capacity ratio, $v/c$	0.47

Capacity, $C_{d,ATS}$ (Equation 15-12) pc/h	1659
Capacity, $C_{d,PTSF}$ (Equation 15-13) pc/h	1700
Percent Free-Flow Speed $PFFS_d$ (Equation 15-11 - Class III only)	79.3
<b>Bicycle Level of Service</b>	
Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	760.5
Effective width, $W_v$ (Eq. 15-29) ft	28.00
Effective speed factor, $S_f$ (Eq. 15-30)	4.79
Bicycle level of service score, BLOS (Eq. 15-31)	12.67
Bicycle level of service (Exhibit 15-4)	F
<b>Notes</b>	
<p>1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific downgrade segments are treated as level terrain.</p> <p>2. If <math>v_d</math> or <math>v_o \geq 1,700</math> pc/h, terminate analysis--the LOS is F.</p> <p>3. For the analysis direction only and for <math>v &gt; 200</math> veh/h.</p> <p>4. For the analysis direction only</p> <p>5. Exhibit 15-20 provides coefficients a and b for Equation 15-10.</p> <p>6. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.</p>	



# Appendix 6

## Projected Four-Lane Highway 2035 – Low Condition

Direction 1 = Northbound/Eastbound Direction

Direction 2 = Southbound/Westbound Direction

MULTILANE HIGHWAYS WORKSHEET(Direction 1)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">X</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Glendive to Savage
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper. (LOS)		<input type="checkbox"/> Des. (N)	
		<input type="checkbox"/> Plan. (vp)	
Flow Inputs			
Volume, V (veh/h)	242	Peak-Hour Factor, PHF	0.81
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	25
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.889
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v <sub>p</sub> (pc/h/ln)	168	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	2.8	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	149.4
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	13.91
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">X</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Glendive to Savage
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Plan. (vp)	
Flow Inputs			
Volume, V (veh/h)	249	Peak-Hour Factor, PHF	0.78
AADT(veh/h)		% Trucks and Buses, P <sub>T</sub>	25
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.889
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v <sub>p</sub> (pc/h/ln)	179	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	3.0	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	159.6
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	13.95
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">X</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Savage to Crane
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Plan. (vp)	
Flow Inputs			
Volume, V (veh/h)	253	Peak-Hour Factor, PHF	0.87
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	23
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.897
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v <sub>p</sub> (pc/h/ln)	162	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	2.7	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	145.4
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	12.39
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">x</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Savage to Crane
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Plan. (vp)	
Flow Inputs			
Volume, V (veh/h)	307	Peak-Hour Factor, PHF	0.84
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	25
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.889
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v <sub>p</sub> (pc/h/ln)	205	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	3.4	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	182.7
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	14.02
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">X</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Crane to Sidney
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Plan. (vp)	
Flow Inputs			
Volume, V (veh/h)	271	Peak-Hour Factor, PHF	0.80
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	19
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.913
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v <sub>p</sub> (pc/h/ln)	185	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	3.1	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	169.4
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_t$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	9.73
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">X</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Crane to Sidney
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Plan. (vp)			
Flow Inputs			
Volume, V (veh/h)	416	Peak-Hour Factor, PHF	0.87
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	19
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.913
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
<u>Operational (LOS)</u>		<u>Design (N)</u>	
Flow Rate, v <sub>p</sub> (pc/h/ln)	261	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	4.3	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	239.1
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	9.91
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">X</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Sidney to Fairview
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Plan. (vp)	
Flow Inputs			
Volume, V (veh/h)	529	Peak-Hour Factor, PHF	0.83
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	17
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.922
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v <sub>p</sub> (pc/h/ln)	345	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	5.8	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	318.7
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	8.82
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">X</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Sidney to Fairview
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - Low
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Plan. (vp)	
Flow Inputs			
Volume, V (veh/h)	523	Peak-Hour Factor, PHF	0.86
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	25
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.889
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v <sub>p</sub> (pc/h/ln)	342	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	5.7	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	304.1
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	14.27
Bicycle level of service (Exhibit 15-4)	F

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# Appendix 6

## Projected Four-Lane Highway 2035 – High Condition

Direction 1 = Northbound/Eastbound Direction

Direction 2 = Southbound/Westbound Direction

MULTILANE HIGHWAYS WORKSHEET(Direction 1)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">X</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Glendive to Savage
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Plan. (vp)			
Flow Inputs			
Volume, V (veh/h)	321	Peak-Hour Factor, PHF	0.81
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	25
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.889
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v <sub>p</sub> (pc/h/ln)	222	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	3.7	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	198.1
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	14.06
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">x</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Glendive to Savage
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Plan. (vp)	
Flow Inputs			
Volume, V (veh/h)	331	Peak-Hour Factor, PHF	0.78
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	25
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.889
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
<u>Operational (LOS)</u>		<u>Design (N)</u>	
Flow Rate, v <sub>p</sub> (pc/h/ln)	238	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	4.0	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	212.2
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_t$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	14.09
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">X</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Savage to Crane
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Plan. (vp)			
Flow Inputs			
Volume, V (veh/h)	336	Peak-Hour Factor, PHF	0.87
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	23
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.897
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
<u>Operational (LOS)</u>		<u>Design (N)</u>	
Flow Rate, v <sub>p</sub> (pc/h/ln)	215	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	3.6	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
Bicycle Level of Service			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	193.1
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	12.54
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">x</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Savage to Crane
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Plan. (vp)			
Flow Inputs			
Volume, V (veh/h)	407	Peak-Hour Factor, PHF	0.84
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	25
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.889
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v <sub>p</sub> (pc/h/ln)	272	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	4.5	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	242.3
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	14.16
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">x</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Crane to Sidney
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Plan. (vp)	
Flow Inputs			
Volume, V (veh/h)	360	Peak-Hour Factor, PHF	0.80
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	19
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.913
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
<u>Operational (LOS)</u>		<u>Design (N)</u>	
Flow Rate, v <sub>p</sub> (pc/h/ln)	246	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	4.1	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	225.0
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	9.88
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">X</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Crane to Sidney
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Plan. (vp)	
Flow Inputs			
Volume, V (veh/h)	552	Peak-Hour Factor, PHF	0.87
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	19
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.913
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v <sub>p</sub> (pc/h/ln)	347	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	5.8	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	317.2
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	10.05
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">X</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Sidney to Fairview
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Plan. (vp)	
Flow Inputs			
Volume, V (veh/h)	661	Peak-Hour Factor, PHF	0.83
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	17
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.922
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v <sub>p</sub> (pc/h/ln)	432	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	7.2	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
Bicycle Level of Service			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	398.2
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	8.93
Bicycle level of service (Exhibit 15-4)	F

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)			
<div style="border: 1px solid black; width: 20px; height: 20px; display: inline-block; margin-bottom: 5px;">X</div>			
General Information		Site Information	
Analyst	David Stoner	Highway/Direction to Travel	MT 16
Agency or Company	DOWL HKM	From/To	Sidney to Fairview
Date Performed	5/1/2012	Jurisdiction	Dawson/Richland County
Analysis Time Period	Peak Hour	Analysis Year	2035 - High
Project Description MT 16 / MT 200 Glendive to Fairview Corridor Planning Study			
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)	
<input type="checkbox"/> Plan. (vp)			
Flow Inputs			
Volume, V (veh/h)	654	Peak-Hour Factor, PHF	0.86
AADT(veh/h)		%Trucks and Buses, P <sub>T</sub>	25
Peak-Hour Prop of AADT (veh/d)		%RVs, P <sub>R</sub>	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2
Calculate Flow Adjustments			
f <sub>p</sub>	1.00	E <sub>R</sub>	1.2
E <sub>T</sub>	1.5	f <sub>HV</sub>	0.889
Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f <sub>LW</sub> (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f <sub>LC</sub> (mi/h)	
Access Points, A (A/mi)	0	f <sub>A</sub> (mi/h)	
Median Type, M		f <sub>M</sub> (mi/h)	
FFS (measured)	60.0	FFS (mi/h)	60.0
Base Free-Flow Speed, BFFS			
Operations		Design	
<u>Operational (LOS)</u>		<u>Design (N)</u>	
Flow Rate, v <sub>p</sub> (pc/h/ln)	427	Required Number of Lanes, N	
Speed, S (mi/h)	60.0	Flow Rate, v <sub>p</sub> (pc/h)	
D (pc/mi/ln)	7.1	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	
<b>Bicycle Level of Service</b>			

Directional demand flow rate in outside lane, $v_{OL}$ (Eq. 15-24) veh/h	380.2
Effective width, $W_v$ (Eq. 15-29) ft	24.00
Effective speed factor, $S_f$ (Eq. 15-30)	5.19
Bicycle level of service score, BLOS (Eq. 15-31)	14.39
Bicycle level of service (Exhibit 15-4)	F

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